

Investigating the Mechanical and Durability Performance of Cement Mortar Incorporated Modified Fly Ash and Ground Granulated Blast Furnace Slag as Cement Replacement Materials

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Abstract— The process of cement manufacturing produces a huge amount of carbon dioxide (CO₂). The utilization of alternative waste materials from various industrial processes as a partial substitution to cement is encouraged due to environmental and specific technical requirements. This strategy will have the potential to reduce cost of cement, conserve energy, and reduce waste volumes. Therefore, the aim of this research is to investigate effect of the replacement of cement with modified fly ash (MFA) and ground granulated blast furnace slag (GGBS) to reach 80% total replacement on mechanical and durability performance of cement mortar. Normal consistency, the initial and final setting times, compressive strength and electrical resistivity of all the ternary mixtures were determined and compared with the control binder. Compressive strength and electrical resistivity were tested at various curing ages of 3, 7, 14, and 28 days. Test results revealed that the normal consistency of the ternary mixtures increased with increasing the GGBS and MFA content, while the initial and final setting time decreased compared to that of control mixture. The results also showed that the compressive strength of all the ternary blends mortars were lower at early and later ages in comparison with control mortar. The reductions in the compressive strengths of the ternary mixtures T40, T60 and T80 compared to the control mixture were approximately 16%, 29% and 37%, respectively at 28 days. The surface electrical resistivity of ternary blends mixtures was higher than the control mixture at all curing ages. The use of GGBS and MFA in the production of cement mortar and concrete can significantly help in reducing the CO₂ emissions of the cement industry and reduce the overall cost of cement.

Keywords— GGBS; MFA; cement replacement; compressive strength; electrical resistivity.

I. INTRODUCTION

Ordinary Portland cement (OPC) is the most common traditional construction material that is used as a binder substance in different civil engineering projects worldwide [1, 2]. However, there is no doubt that the cement industry has a negative environmental impact due to the emission of Carbon dioxide (CO₂) which contributes to the phenomenon

called greenhouse gasses (GHG) [3-5]. It has been reported by many researchers that cement industry is energy intensive with approximately 15% of global energy consumption. Moreover, it consumes 1.5 tonne of raw materials and produces about 1 tonne of CO₂ emission for each single tonne of manufactured cement [6-8].

Therefore, looking for alternative materials to cement has become a vital issue to reduce the damage caused by the cement industry. By-products or waste materials can be promising alternatives to cement for production of new binders for the use in different applications in civil engineering projects.

Huge quantities of wastes and industrial by-product are resulted from different industrial sectors globally. These materials represent another environmental issue due to the costs of transporting, lack of lands for depositing and the leakage of potential hazardous compounds that may be available within the chemical compositions of the by-products [2]. Therefore, in order to overcome this problem, the reuse of such materials as cement replacement can contribute in both the depositing issue of such materials as well as to reduce the use of cement, which reflects a reduction of cement manufacturing impact [7].

Although the use of a combination of fly ash (FA) and ground granulated blast furnace slag (GGBS) as cement replacement has been extensively investigated in numerous research articles, however, the use of a combination of modified fly ash (MFA) and GGBS as cement replacement material is a novel approach. Therefore, this paper presents the results of experimental work to investigate the mechanical and durability performance of cement mortar incorporated MFA and GGBS as cement replacement materials.

II. MATERIALS

A. Sand

Building sand passing from sieve size 3.35mm was used to prepare the mortar samples. This type of sand is identified in the British standard BS EN 196-1[11]. The particle size distribution of the sand is presented in figure 1.

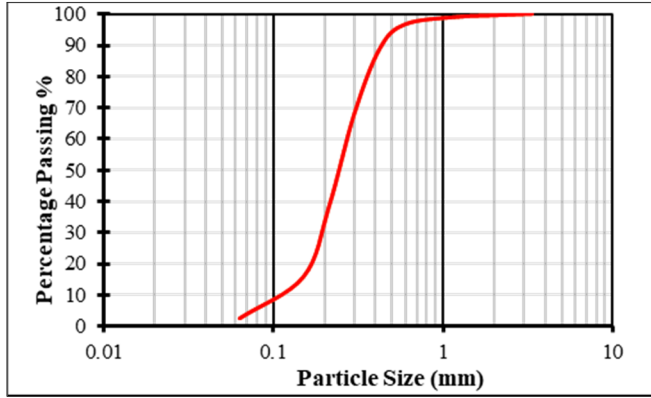


Fig. 1. Particle size distribution of the sand.

B. Water

Tap water was used in this study to prepare all samples tested in this study.

C. Binder materials

Ordinary Portland cement (OPC), modified fly ash (MFA) and ground granulated blast furnace slag (GGBS) were used as binder materials in the production of cement mortar. The cement used in this study was OPC type CEM-II/A/LL 32.5-N and it was supplied by the CEMEX Quality Department, Warwickshire, UK. The GGBS was provided by the Hanson Heidelberg Cement Group, Scunthorpe, UK. The MFA is FA that mixed with a combination of supplementary hydraulic cementitious materials. The MFA was supplied by Cenin Ltd, Bridgend, UK.

The chemical compositions of OPC, GGBS and MFA were determined using the X-Ray fluorescence (XRF) analysis type Shimadzu EDX-720. The results obtained from this test are listed in Table I. Regarding the classification of MFA, the CaO content in MFA (32.86%) makes it be classified as a fly ash class C according to the British Standard BS EN 450:1995 [12]. Fly ashes class C has sufficient CaO content, which can exhibit a hardening property without cement when they are added to the water [10]. Regarding to GGBS, it has a significant proportions of most principle oxides (Ca, Si, Mg and Al) and its $(CaO+MgO/SiO_2)$ is more than 1.0 which is agrees with BS EN 197-1:2000 requirements for granulated blast furnace slag [14].

The particle size distribution (PSD) and specific surface area (SSA) are important physical tests that provided information about the fineness of the binder materials. Figure 2 shows the PSD of the OPC, GGBS and MFA as obtained from the laser particle size analyser.

TABLE I. CHEMICAL COMPOSITION OF OPC, GGBS AND MFA

| Item | OPC | GGBS | MFA |
|----------------------------------|-------|-------|-------|
| CaO % | 65.21 | 42.51 | 32.86 |
| SiO ₂ % | 24.56 | 41.06 | 43.77 |
| Al ₂ O ₃ % | 1.70 | 5.12 | 4.94 |
| Fe ₂ O ₃ % | 1.64 | - | 6.96 |
| MgO % | 1.30 | 4.25 | 4.91 |
| Na ₂ O % | 1.34 | 3.09 | 2.92 |
| K ₂ O % | 0.82 | 0.69 | 2.26 |
| SO ₃ % | 2.62 | 1.27 | 1.30 |
| TiO ₂ % | - | 0.98 | 0.08 |
| LOI % | 0.28 | 0.37 | 2.41 |
| pH | 12.73 | 11.02 | 10.68 |
| Specific Gravity | 2.94 | 3.05 | 2.71 |

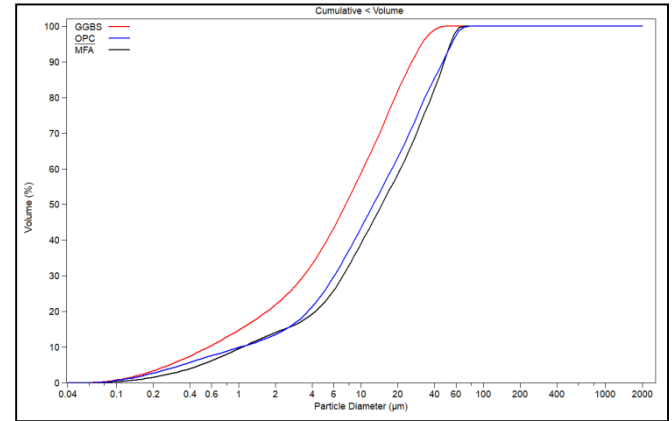


Fig. 2. Cumulative particle size distribution of OPC, GGBS and MFA.

Additionally, Table 4.2 shows the differences in d_{50} , median and SSA for the OPC, GGBS and MFA.

TABLE II. DIFFERENT IN D_{50} , MEDIAN AND SSA OF THE BINDER MATERIALS

| Mix Id | OPC (%) | GGBS (%) | MFA (%) |
|--------------------------------|---------|----------|---------|
| d_{50} (µm) | 9.43 | 5.40 | 10.68 |
| Median (µm) | 12.66 | 7.52 | 14.84 |
| Blain SSA (cm ² /g) | 4296 | 6931 | 4645 |

The PSD and SSA of the binder materials (OPC, GGBS and MFA) have a significant effect on the compressive strength of the mortar and concrete. The finer the particles of the material used as partial replacement to cement in mortar and concrete production, the higher the compressive strength obtained [15, 16]. It can be seen from the particle size distribution chart in figure 2 that GGBS has finer particles relative to the other materials. This means that the GGBS has a higher pozzolanic reactivity than the other materials as it has a higher SSA [1, 17].

III. METHODOLOGY

A. Mixing proportions

Three different ternary mixtures prepared from mixing OPC, GGBS and MFA using different proportion in addition to the reference binder (100% OPC) was considered in this study as illustrated in Table III. The water to binder (W/B) and the sand to binder (S/B) ratios were maintained constant as 0.4 and 1:2.5, respectively for all mortar mixtures.

TABLE III. MIXING PROPORTION OF THE TERNARY MIXTURES

| Mix Id | OPC (%) | GGBS (%) | MFA (%) |
|---------|---------|----------|---------|
| Control | 100 | 0 | 0 |
| T40 | 60 | 20 | 20 |
| T60 | 40 | 30 | 30 |
| T80 | 20 | 40 | 40 |

B. Samples' preparation

After weighing the required weight of sand and binder, the materials were mixed using Hobart N50-110 mixer for 5 minutes. After that, a tap water was added and another mixing process for 5 minutes was applied to form the cement mortar. For the samples of the compression strength test, the mortars were casted in moulds with dimensions of 40 X 40 X 160mm while for the electrical resistivity test; the mortars were casted in cylindrical moulds with diameter of 100 mm and height of 200 mm.

Regarding the curing conditions, the samples tested for compressive strength and electrical resistivity tests were subjected to a wide range of curing period covered the short periods (3 and 7 days) and later curing periods (14 and 28 days) in order to investigate the effect of curing time on the evolution of the compressive strength and electrical resistivity. The samples were cured at the ambient temperature and were kept in a plastic tank filled with a tap water until reach the designed age of curing.

C. Testing programme

The experiment utilised in this study comprised the physical, mechanical and durability properties of the paste and mortars prepared using binders produced from OPC replaced with GGBS and MFA at different percentages.

D. Consistency and Setting Times

The consistency test depends mainly on the water to binder ratio, fineness and rate of hydration reactions of the binder [18]. By using the Vicat apparatus, the consistency test was done first on paste samples prepared from OPC replaced with the GGBS and MFA at different percentages. This test was conducted to evaluate the best water/binder ratio for each prepared mixture. This ratio is very important; it is required to prepare the paste, and mortar samples for other experiments. After the identification of the optimum water/binder ratio, the determination of hardening times (initial and final setting times) was carried out using the same apparatus (Vicat). According to Naik et al. [19], the test of setting times helps to get indications about the hardening properties and the availability of sufficient hardness of the binder. The test of the consistency and setting times was carried out in accordance with BS EN 196-3[20]

E. Compressive strength

Compressive strength testing was conducted to assess the mechanical performance of the cement mortars according to BS EN 196-1[21]. Three samples of dimensions 40 x 40 x 160 mm were prepared for each mixing proportion and the test was conducted after 3, 7, 14 and 28 days of curing.

F. Electrical resistivity

One of the most important features of concrete is the long-term performance against the up normal or severe conditions. This performance is known as the durability. Since the electrical resistivity of the concrete plays a very big role in predicting the corrosion of the reinforced concrete [22], this test was utilised in this study. A Resipod Proceq meter was used to perform this test. The electrical resistivity was measured in accordance with AASHTO T358 [23]. This test is directly connected to the possibility of corrosion of concrete and mortar due to the diffusion of chloride [18, 24]. Three cylinders with a diameter of 100 mm and a height of 200 mm were prepared and tested after 3, 7, 14 and 28 of curing. The readings were obtained 8 times for each specimen and the averages of 24 readings were taken to represent the final values for electrical resistivity.

IV. RESULTS AND DISCUSSION

A. Consistency and setting time

Table IV illustrates the measured consistency and setting times (initial and final) of the pastes prepared from binders produced from OPC, GGBS and MFA used in this study with different percentages of OPC replacement. The corresponding results of the consistency and setting times of the control binder (OPC) are also listed in Table IV for the purpose of comparison. Table IV shows that the consistency of control mixture and ternary mix with 20% MFA and 20% GGBS replacement levels (T40) is 33%, while that of the ternary mixtures containing 30% and 40% of MFA and GGBS replacement levels (T60 and T80) shows increased values.

The consistency of the ternary mixtures contained (MFA+GGBS) varies between 33% and 35%, depending on ground conditions of binder materials. The increased values in the consistencies of ternary mixtures (T40 and T60) are due to high fineness and the high SSA of MFA and GGBS particles [18, 25, 26]. Generally, waste and/or by-product materials require more amount of water to get desire consistency, which has been reported in previous studies [26, 27].

TABLE IV. RESULTS OF CONSISTENCY AND SETTING TIMES TESTS

| Mix Id | Consistency % | Initial setting time (min) | Final setting time (min) |
|---------|---------------|----------------------------|--------------------------|
| Control | 33% | 275 | 292 |
| T40 | 33% | 261 | 285 |
| T60 | 35% | 225 | 243 |
| T80 | 35% | 172 | 186 |

The initial and final setting time of control and ternary mixtures are presented in the Table IV. The results shows that ternary binders tend to reduce the initial and final setting time of mortar compared to control mixture. The reduction in the initial and final setting time was increased with

increasing the percentages of replacement of cement with GGBS+MFA. This reduction is mainly attributed to the presence of binder materials with high SSA (MFA and GGBS) that reduces the amount of free water available in the mixture. These results are generally consistent with previous findings by Sadique et al. [26] and Salih et al. [28]

B. Compressive strength

The compressive strength is the most important design parameter for any type of concrete structures. In this regard, strength development of mortars made with OPC, GGBS and MFA for various ages of curing are presented in figure 3.

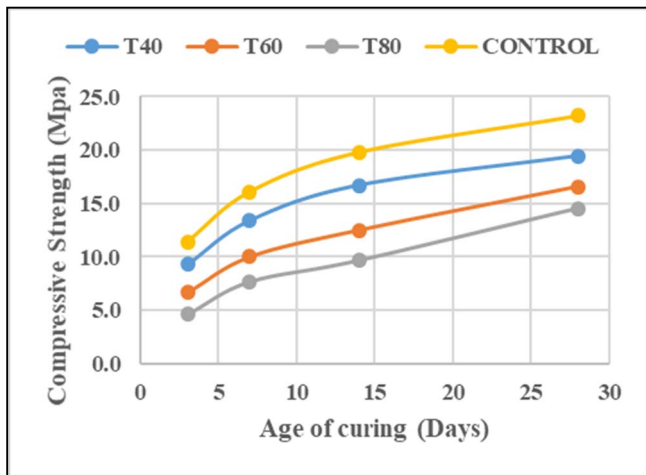


Fig. 3. Compressive strength development of different ternary blends at different ages

As expected, the strength of all mixtures increases continuously with time. It is obvious from figure 3 that all the ternary blended mortars containing (MFA+GGBS) exhibited lower strength from 3 days onwards with increased replacement levels in comparison with the control mixture.

At the age of 3 days, the mixtures T40, T60 and T80 have showed about 82%, 58% and 41% of the compressive strength of the control mixture. Increasing the age of curing to 7 days resulted in improvement in the compressive strength of the ternary mixtures relative to the control mixture. The 7 days compressive strength of T40, T60 and T80 were 83%, 62% and 48%, respectively in comparison with the control mixture. This reduction in the compressive strength during the first week with increasing the percentages of GGBS and MFA could be attributed to the slow acquisition of strength for mixtures with high volume of GGBS and MFA at initial curing ages [18, 29, 30].

After 14 days of curing, the compressive strength of all the ternary mixtures have been improved by about 25% relative to that at the age of 7 days, while the improvement in the control mixture was about 23%. Additionally, the mixtures T40, T60 and T80 have provided about 84%, 63% and 49% of the compressive strength of the control mixture, respectively.

At the age of 28 days, the compressive strength of the ternary mixture T40 was 19.4 MPa that is about 84% of the compressive strength of the control mixture (23.2 MPa) with improvement of 16% relative to the 14 days compressive strength. For the ternary mixtures (T60 and T80) there was a significant improvement in the compressive strength after 28 days relative to that at the age of 14 days with about 132% and 150%, respectively. However, the ternary mixtures T60

and T80 provided about 71% and 63% of the compressive strength of the control mixture respectively.

C. Electrical resistivity

The results of the electrical resistivity of the mortars over the time of curing are presented in figure 4. It can be easily recognised that incorporating both GGBS and MFA in this study improved the electrical resistivity of the produced binders and their mortars. This reflects an enhancing of the durability performance. Unlike what was obtained from the compressive strength, the electrical resistivity for all mortars contained GGBS and MFA increased with the increase of the GGBS and MFA content that replaced OPC for the mortars tested at all curing ages (3, 7, 14 and 28 days) as shown in Fig. 4. The results also showed that the control binder (OPC) indicated an electrical resistivity similar to that of the T40 mixture only after 3 days of curing. For longer curing time, the ternary binders indicated electrical resistivity higher than that for the control binder.

This improvement in surface electrical resistivity of ternary mixtures lead to increase the resistance to chloride penetration compared to the control mortar [24, 31]. Thus, improved resistance of ternary blends mixtures to the chloride penetration is mainly due to the incorporation of MFA and GGBS that lead to reduce diffusivity of chloride ions. This is due to the formation of a dense microstructure in the interfacial transition zone. It also increases the density and calcium silicate hydrate (C-S-H) gel of the cement paste generated during cement hydration, which in turn significantly improves the strength and durability of the mortar, as observed by other authors [18, 24, 32].

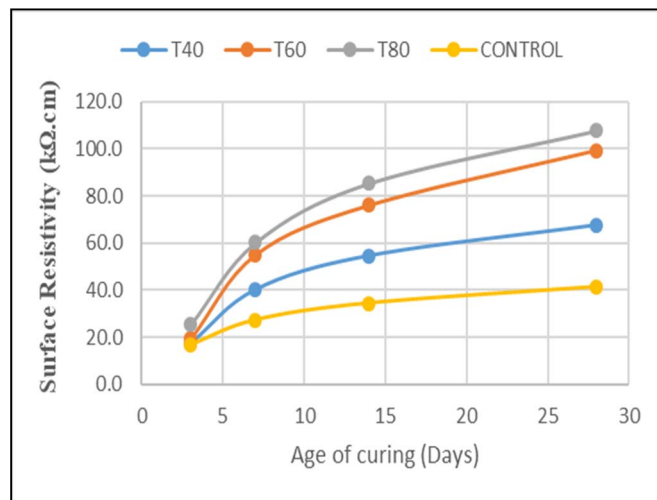


Fig. 4. Electrical resistivity of different ternary blends at different ages

According to the AASHTO T 358 [23] classification, both the control mixture and ternary mixtures T40 and T60 have showed moderate chloride ion penetrability while the ternary mixture T80 has showed low chloride ion penetrability after 3 days of curing. All the ternary mixtures have showed very low chloride ion penetrability at the age of 7 days onwards with a significant improvement in the electrical resistivity and increasing the GGBS and MFA content. On the other hand, the control mixture has showed low chloride ion penetrability after 7 to 14 days of curing and very low chloride ion penetrability at the age of 28 days.

V. CONCLUSION

Based on the results obtained from the present study, we have drawn the following conclusions:

- The consistency of the new ternary mixtures contained MFA+GGBS varies between 33% and 35%, depending on ground conditions of waste materials. Consistency of control mixture and ternary mixture T40 was found to be 33%, while that of the ternary mixtures T60 and T80 was found to have a consistency of 35%.

- All the ternary mixtures tend to reduce the initial and final setting time of paste in comparison with control mixture. The setting time values are arranged from the highest to the lowest value as follows: control, T40, T60 and T80.

- All the ternary mortars mixtures containing MFA+GGBS exhibited a reduction in the compressive strength at all curing ages with increasing the replacement levels in comparison with the control mortar.

- At all curing ages, all the ternary mixtures have showed better electrical resistivity performance than the control mixture and the improvement in the electrical resistivity was proportional with increasing the GGBS and MFA content.

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